



U.S. Department
of Transportation
Federal Highway
Administration

Publication No. FHWA NHI-01-031
May 2002

NHI Course No. 132031

Subsurface Investigations

— Geotechnical Site Characterization

Reference Manual



National Highway Institute

NOTICE

The contents of this report reflect the views of the authors, who are responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect policy of the U.S. Department of Transportation. This report does not constitute a standard, specification, or regulation. The United States Government does not endorse products or manufacturers. Trade or manufacturer's names appear herein only because they are considered essential to the object of this document.

Technical Report Documentation Page

1. REPORT NO. FHWA-NHI-01-031	2. GOVERNMENT ACCESSION NO.	3. RECIPIENT'S CATALOG NO.	
4. TITLE AND SUBTITLE Subsurface Investigations (Geotechnical Site Characterization)		5. REPORT DATE May 2002	6. PERFORMING ORGANIZATION CODE
7. AUTHOR(S) Paul W. Mayne, Ph.D., P.E., Barry R. Christopher, Ph.D., P.E. and Jason DeJong, Ph.D.		8. PERFORMING ORGANIZATION REPORT NO.	
9. PERFORMING ORGANIZATION NAME AND ADDRESS Ryan R. Berg & Associates, Inc. 2190 Leyland Alcove Woodbury, MN 55125		10. WORK UNIT NO.	11. CONTRACT OR GRANT NO. DTFH61-00-T-25043
12. SPONSORING AGENCY NAME AND ADDRESS National Highway Institute Federal Highway Administration U.S. Department of Transportation Washington, D.C.		13. TYPE OF REPORT & PERIOD COVERED	
		14. SPONSORING AGENCY CODE	
15. SUPPLEMENTARY NOTES FHWA Technical Consultants: J.A. DiMaggio, P.E. and P. Osborn, P.E. COTR - L. Jones <i>This manual is an update and revision of FHWA-HI-97-021 prepared by Parsons Brinckerhoff Quade & Douglas, Inc, in association with Woodward-Clyde Consultants; Principal Investigator: George Munfakh; Authors: Ara Arman, Naresh Samtani, Raymond Castelli and George Munfakh; FHWA Technical Consultants: J.A. DiMaggio, A. Muñoz, A. Kilian, and P. Osborn</i>			
16. ABSTRACT This manual is the reference text used for the FHWA NHI course No. 13231 on Subsurface Investigations and reflects current practice for such. The planning, execution, and interpretation of geotechnical site explorations in natural soil and rock are presented with regard to the design and construction of transportation facilities. The role of the geotechnical engineer in subsurface investigation, exploration methods, equipment types and their suitability are discussed. Various in-situ tests are presented, including cone penetration, dilatometer, pressuremeter, vane, and standard penetration. Rotary drilling and rock coring are reviewed in terms of the proper handling, transportation, and storage of soil and rock samples for laboratory testing. Geophysical wave and electromagnetic methods are covered. Laboratory index, strength, and stiffness testing are reviewed in complement to the field testing program. Geomaterial characterization requires the interpretation and correlation of engineering properties from the acquired field and lab measurements. The results are summarized in a geotechnical report with available geological, topographical, hydrological, and geotechnical data collected towards the analysis and design of earthwork structures and foundation design.			
17. KEY WORDS Subsurface, investigation, geomaterials, subgrade, exploration, drilling, coring, sampling, soil, rock, field testing, in-situ, laboratory testing, geophysics, cone penetrometer, vane, groundwater, geotechnical report, transportation, tunnels, slopes, highways, bridges.		18. DISTRIBUTION STATEMENT No restrictions.	
19. SECURITY CLASSIF. Unclassified	20. SECURITY CLASSIF. Unclassified	21. NO. OF PAGES 300	22. PRICE

[Blank]

PREFACE

This module is the first in a series of twelve modules that constitute a comprehensive training course in geotechnical and foundation engineering. Sponsored by the National Highway Institute (NHI) of the Federal Highway Administration (FHWA), the training course is given at different locations in the U.S. The intended audience includes civil engineers and engineering geologists involved in the design and construction of transportation facilities. This manual is designed to present the latest methodologies in the planning, execution and interpretation of the various subsurface investigation methods, and the development of appropriate soil and rock parameters for engineering applications.

The authors have made every effort to present the general state of the practice of subsurface exploration and geotechnical site characterization. It is understood that the procedures discussed in the manual are subject to local variations. It is important, therefore, for the reader to become thoroughly familiar with the local practices as well. This guide focuses on the scope and specific elements of typical geotechnical investigation programs for design and construction of highways and related transportation facilities. Considering the broad scope and fundamental importance of this subject, this manual on subsurface investigations is organized as follows:

- ' Chapters 1 through 6 discuss various aspects of field investigations, including soil borings, augering, rock coring, sampling, in-situ testing, and geophysical exploration methods.
- ' Chapters 7 and 8 discuss laboratory testing of soil and rock materials.
- ' Chapters 9 and 10 present interpretation procedures for soil and rock properties.
- ' Chapters 11 and 12 address issues related to data management and interpretation, including evaluation and synthesis of the field and laboratory test data, development of soil and rock design parameters, and the presentation of investigation findings in geotechnical reports.
- ' Chapter 13 contains a list of cited references for further details & information.
- ' Appendix A contains information on health and safety issues.
- ' Appendix B lists names and websites of soil & rock drilling and in-situ testing equipment manufacturers, distributors, and service companies.

This manual is not intended to be an exclusive reference on subsurface investigations and it is highly recommended that the references given in Chapter 13 be made part of the reader's library and reviewed in detail. Two important references are the *Manual on Subsurface Investigations* by AASHTO (1988) and the FHWA Manual *Evaluation of Soil and Rock Properties* (Geotechnical Engineering Circular No. 5, 2001). Finally, this manual is developed to be used as a living document. After attending the training session, it is intended that the participant will use it as a manual of practice in everyday work. Throughout the manual, attention is given to ensure the compatibility of its content with those of the participants manuals prepared for the other training modules. Special efforts are made to ensure that the included material is practical in nature and represents the latest developments in the field.

SI CONVERSION FACTORS				
APPROXIMATE CONVERSIONS FROM SI UNITS				
Symbol	When You Know	Multiply By	To Find	Symbol
LENGTH				
mm	millimeters	0.039	inches	in
m	meters	3.28	feet	ft
m	meters	1.09	yards	yd
km	kilometers	0.621	miles	mi
AREA				
mm ²	square millimeters	0.0016	square inches	in ²
m ²	square meters	10.764	square feet	ft ²
ha	hectares	2.47	acres	ac
km ²	square kilometers	0.386	square miles	mi ²
VOLUME				
ml	millimeters	0.034	fluid ounces	fl oz
l	liters	0.264	gallons	gal
m ³	cubic meters	35.71	cubic feet	ft ³
m ³	cubic meters	1.307	cubic yards	yd ³
MASS				
g	grams	0.035	ounces	oz
kg	kilograms	2.205	pounds	lb
TEMPERATURE				
°C	Celsius	1.8 C + 32	Fahrenheit	°F
WEIGHT DENSITY				
g/cc	grams per cubic centimeter	62.4	poundforce /cubic foot	pcf
kN/m ³	kilonewton /cubic meter	6.36	poundforce /cubic foot	pcf
FORCE and LOAD				
N	newtons	0.225	poundforce	lbf
kN	kilonewtons	225	poundforce	lbf
kg	kilogram (force)	2.205	poundforce	lbf
MN	meganewtons	112.4	tons (force)	t
PRESSURE and STRESS*				
kPa*	kilopascals	0.145	poundforce /square inch	psi
kPa	kilopascals	20.9	poundforce /square foot	psf
MPa	megapascal	10.44	tons per square foot	tsf
kg/cm ²	kilograms per square cm	1.024	tons per square foot	tsf

*Notes: 1 kPa = kN/m² = one kilopascal = one kilonewton per square meter.

For dimensionless graphs and equations, a reference stress of one atmosphere can be used, such that $\sigma_a = p_{atm} = 1$ bar = 100 kPa . 1 tsf . 1 kg/cm².

SUBSURFACE INVESTIGATIONS

TABLE OF CONTENTS

	Page
Preface	i
Units Conversions	ii
Table of Contents	iii
LIST OF TABLES	xiii
LIST OF FIGURES	xv
LIST OF NOTATIONS	xxii
1.0 Introduction	1-1
1.1 Scope of this Manual	1-1
1.2 Geotechnical Engineer's Role in Subsurface Exploration	1-2
2.0 Project Initiation	2-1
2.1 Project Type	2-1
2.1.1 New Construction	2-1
2.1.2 Rehabilitation Projects	2-2
2.1.3 Contaminated Sites	2-3
2.2 Existing Data Sources	2-4
2.3 Site Visit/plan-in-hand	2-6
2.4 Communication with Designers/Project Managers	2-6
2.5 Subsurface Exploration Planning	2-8
2.5.1 Types of Investigation	2-8
• Remote Sensing	2-8
• Geophysical Investigations	2-10
• Disturbed Sampling	2-10
• In-Situ Investigation	2-10
• Undisturbed Sampling	2-10
2.5.2 Frequency and Depth of Borings	2-10
2.5.3 Boring Locations and Elevations	2-12
2.5.4 Equipment	2-12
2.5.5 Personnel and Personal Behavior	2-15
2.5.6 Plans and Specifications	2-15
2.6 Standards and Guidelines	2-15
3.0 Drilling and Sampling of Soil and Rock	3-1
3.1 Soil Exploration	3-1
3.1.1 Soil Drilling	3-1

• Continuous Flight Augers	3-1
• Hollow-Stem Augers	3-2
• Rotary Wash Borings	3-5
• Bucket Auger Borings	3-8
• Area Specific Methods	3-9
• Hand Auger Borings	3-9
• Exploration Pit Excavation	3-9
• Logging Procedures	3-10
• Photography	3-10
3.1.2 Soil Samples	3-10
• Disturbed Samples	3-10
• Undisturbed Samples	3-10
3.1.3 Soil Samplers	3-11
• Split Barrel Sampler	3-11
• Thin Wall Sampler	3-14
• Piston Sampler	3-15
• Pitcher Tube Sampler	3-16
• Denison Sampler	3-18
• Modified California Sampler	3-18
• Continuous Soil Sampler	3-18
• Other Soil Samplers	3-19
• Bulk Samples	3-19
• Block Samples	3-19
3.1.4 Sampling Interval and Appropriate Type of Sampler	3-20
3.1.5 Sample Recovery	3-20
3.1.6 Sample Identification	3-20
3.1.7 Relative Strength Tests	3-21
3.1.8 Care and Preservation of Undisturbed Soil Samples	3-21
3.2 Exploration of Rock	3-22
3.2.1 Rock Drilling and Sampling	3-22
3.2.2 Non-Core (Destructive) Drilling	3-23
3.2.3 Types of Core Drilling	3-23
• Coring Bits	3-25
• Drilling Fluid	3-26
3.2.4 Observation During Core Drilling	3-26
• Drilling Rate/Time	3-26
• Core Photographs	3-26
• Rock Classification	3-27
• Recovery	3-27
• Rock Quality Designation (RQD)	3-27
• Length Measurements of Core Pieces	3-27
• Assessment of Soundness	3-28
• Drilling Fluid Recovery	3-29
• Core Handling and Labeling	3-29
• Care and Preservation of Rock Samples	3-31
3.2.5 Geologic Mapping	3-31
3.3 Boring Closure	3-32
3.4 Safety Guidelines for Geotechnical Borings	3-32
3.5 Common Drilling Errors	3-35

4.0	Boring Log Preparation	4-1
4.1	General	4-1
4.2	Project Information	4-9
4.3	Boring Locations and Elevations	4-9
4.4	Stratigraphy Identification	4-9
4.5	Sample Information	4-10
4.6	Soil Description and Soil Classification	4-10
4.6.1	Soil Description	4-10
	• Consistency and Apparent Density	4-11
	• Water Content (Moisture)	4-11
	• Color	4-12
	• Type of Soil	4-13
	< Coarse-Grained Soils (Gravel and Sand)	4-13
	- Feel and Smear Test	4-13
	- Sedimentation Test	4-14
	- Visual Characteristics	4-14
	< Fine-Grained Soils	4-15
	- Shaking (Dilatancy) Test	4-15
	- Dry Strength Test	4-15
	- Thread Test	4-15
	- Smear Test	4-15
	• Highly Organic Soils	4-15
	• Minor Soil Type(s)	4-17
	• Inclusions	4-17
	• Layered Soils	4-17
	• Geological Name	4-18
4.6.2	Soil Classification System	4-18
	• The Unified Classification System	4-18
	• Classification of Coarse-Grained Soils	4-19
	• Classification of Fine-Grained Soils	4-23
4.6.3	AASHTO Soil Classification System	4-26
4.7	Logging Procedures for Core Drilling	4-28
4.7.1	Description of Rock	4-28
4.7.2	Rock Type	4-29
4.7.3	Color	4-29
4.7.4	Grain Size and Shape	4-29
4.7.5	Stratification/Foliation	4-29
4.7.6	Mineral Composition	4-32
4.7.7	Weathering and Alteration	4-32
4.7.8	Strength	4-32
4.7.9	Hardness	4-32
4.7.10	Rock Discontinuity	4-33
4.7.11	Fracture Description	4-34
5.0	In-Situ Testing	5-1
5.1	Standard Penetration Test (SPT)	5-2
5.2	Cone Penetration Testing (CPT)	5-5
	• Piezocone Penetration Testing (CPTu)	5-6
	• Baseline Readings	5-8

• Routine CPTu Operations	5-8
• CPT Profiles	5-9
5.3 Vane Shear Test (VST)	5-10
• Undrained Strength and Sensitivity	5-11
• Field Vane Results	5-13
• Vane Correction Factor	5-14
5.4 Flat Plate Dilatometer Testing (DMT)	5-16
5.5 Pressuremeter Testing (PMT)	5-19
5.6 Special Probes and In-Situ Tests	5-22
5.7 Geophysical Methods	5-23
5.7.1 Mechanical Waves	5-23
5.7.2 Seismic Refraction (SR)	5-25
5.7.3 Crosshole Tests (CHT)	5-26
5.7.4 Downhole Tests (DHT)	5-27
5.7.5 Surface Waves (SASW)	5-30
5.7.6 Electromagnetic Waves	5-32
• Ground Penetrating Radar	5-32
• Electrical Resistivity (ER) Survey	5-34
• Electromagnetic (EM) Techniques	5-35
• Magnetic Surveys	5-36
5.8 Summary on In-Situ and Geophysical Methods	5-36
 6.0 Groundwater Investigations	6-1
6.1 General	6-1
6.2 Determination of Groundwater Levels and Pressures	6-1
6.2.1 Information on Existing Wells	6-1
6.2.2 Open Borings	6-2
6.2.3 Observation Wells	6-2
6.2.4 Water Level Measurements	6-3
• Chalked Tape	6-4
• Tape with a Float	6-4
• Electric Water-Level Indicator	6-4
• Data Loggers	6-4
6.3 Field Measurement of Permeability	6-5
6.3.1 Seepage Tests	6-6
• Falling Water Level Method	6-7
• Rising Water Level Method	6-7
• Constant Water Level Method	6-7
6.3.2 Pressure (“Packer”) Test	6-8
6.3.3 Pumping Tests	6-10
6.3.4 Slug Tests	6-15
6.3.5 Piezocone Dissipation Tests	6-16
 7.0 Laboratory Testing for Soils	7-1
7.1 General	7-1
7.1.1 Weight-Volume Concepts	7-1
7.1.2 Load-Deformation Process in Soils	7-2
7.1.3 Principle of Effective Stress	7-3
7.1.4 Overburden Stress	7-3

7.1.5	Selection & Assignment of Tests	7-4
7.1.6	Visual Identification of Soils	7-7
7.1.7	Index Properties	7-8
	• Moisture Content	7-8
	• Specific Gravity	7-8
	• Unit Weight	7-9
	• Sieve Analysis	7-10
	• Hydrometer Analysis	7-11
	• Atterberg Limits	7-11
	• Moisture-Density (Compaction) Relationship	7-13
	• Classification of Soils	7-14
	• Corrosivity of Soils	7-14
	• Resistivity	7-15
	• Organic Content of Soils	7-15
7.1.8	Strength Tests	7-16
	Total or Effective Stress Analysis	7-16
	• Unconfined Compressive Strength of Soils	7-18
	• Triaxial Strength (TX)	7-19
	• Direct Shear (DS)	7-22
	• Direct Simple Shear (DSS) Test	7-22
	• Resonant Column	7-24
	• Miniature Vane	7-26
	• California Bearing Ratio (CBR)	7-26
	• R-Value Test	7-27
	• Resilient Modulus	7-27
7.1.9	Permeability	7-28
7.1.10	Consolidation	7-30
	• One-Dimensional Compression (Consolidation)	7-30
	• Swell Potential of Clays	7-33
	• Collapse Potential of Soils	7-33
7.2	Quality Assurance for Laboratory Testing	7-34
7.2.1	Storage	7-34
7.2.2	Sample Handling	7-34
7.2.3	Specimen Selection	7-34
7.2.4	Equipment Calibration	7-35
7.2.5	Pitfalls	7-35
7.3	Selection and Assignment of Tests	7-37
8.0	Laboratory Testing for Rocks	8-1
8.1	Introduction	8-1
8.2	Laboratory Tests	8-1
8.2.1	Strength Tests	8-1
	• Point-Load Strength Test	8-3
	• Uniaxial Compression Test	8-4
	• Split Tension (Brazilian) Test	8-5
	• Direct Shear Strength of Rock	8-6
8.2.2	Durability	8-7
	• Slake Durability	8-8
	• Soundness of Riprap	8-9

8.2.3	• Durability Under Freezing and Thawing	8-9
	Strength-Deformation Characteristics	8-10
	• Elastic Modulii	8-10
	• Ultrasonic Testing	8-11
8.3	Quality Assurance for Laboratory Testing	8-12
	8.3.1 Cautions	8-12
 9.0	 Interpretation of Soil Properties	 9-1
9.1	Introduction	9-1
9.2	Compositional and Classification	9-2
	9.2.1 Soil Classification and Geostratigraphy	9-2
	9.2.2 Soil Classification by Soil Sampling & Drilling	9-2
	9.2.3 Soil Classification by Cone Penetration Testing	9-4
	9.2.4 Soil Classification by Flat Dilatometer	9-6
9.3	Density	9-6
	9.3.1 Unit Weight	9-6
	9.3.2 Relative Density Correlations for Sands	9-8
9.4	Strength and Stress History	9-13
	9.4.1 Drained Friction Angles of Sands	9-13
	9.4.2 Preconsolidation Stress of Clays	9-17
	9.4.3 Undrained Strength of Clays and Silts	9-22
	9.4.4 Lateral Stress State	9-25
9.5	Stiffness and Deformation Parameters	9-27
	9.5.1 Small-Strain Shear Modulus	9-29
	9.5.2 Modulus Reduction	9-30
	9.5.3 Direct and Indirect Assessments of G_0	9-32
9.6	Flow Properties	9-36
	9.6.1 Monotonic Dissipations	9-36
	9.6.2 Dilatory Dissipations	9-40
9.7	Nontextbook Materials	9-42
 10.0	 Interpretation of Rock Properties	 10-1
10.1	Introduction	10-1
10.2	Intact Rock Properties	10-5
	10.2.1 Specific Gravity	10-5
	10.2.2 Unit Weight	10-5
	10.2.3 Ultrasonic Velocities	10-6
	10.2.4 Compressive Strength	10-8
	10.2.5 Direct and Indirect Tensile Strength	10-9
	10.2.6 Elastic Modulus of Intact Rock	10-11
10.3	Operational Shear Strength	10-15
10.4	Rock Mass Classifications	10-18
	10.4.1 Rock Mass Rating System (RMR)	10-20
	10.4.2 NGI Q-Rating	10-20
	10.4.3 Geological Strength Index (GSI) System	10-22
10.5	Rock Mass Strength	10-24
10.6	Rock Mass Modulus	10-26
10.7	Foundation Resistances	10-27

10.7.1	Allowable Bearing Stress for Foundations	10-27
10.7.2	Side Resistances for Drilled Shafts	10-29
10.8	Additional Rock Mass Parameters	10-30
11.0	Geotechnical Reports	11-1
11.1	Types of Reports	11-1
11.1.1	Geotechnical Investigation Reports	11-1
11.1.2	Geotechnical Design Reports	11-2
11.1.3	GeoEnvironmental Reports	11-5
11.2	Data Presentation	11-5
11.2.1	Borings Logs	11-5
11.2.2	Test Location Plans	11-6
11.2.3	Subsurface Profiles	11-8
11.3	Limitations	11-9
12.0	Contracting of Geotechnical Subsurface Exploration	12-1
12.1	Drilling and Testing Specifications	12-1
13.0	References	13-1
Appendix A	Safety Guidelines for Drilling into Soil and Rock and Health and Safety Procedures for Entry into Borings	A-1
Appendix B	Websites: Geotechnical Equipment Suppliers and Service Testing Companies	B-1

[Blank]

LIST OF TABLES

Table 2-1.	General Guidelines for Geotechnical Field Inspectors	2-9
Table 2-2.	Minimum Requirements for Boring Depths	2-11
Table 2-3.	Guidelines for Boring Layout	2-13
Table 2-4.	List of Equipment for Field Explorations	2-14
Table 2-5.	Frequently-used Standards for Field Investigations	2-16
Table 3-1.	Dimensions of Common Hollow-stem Augers	3-3
Table 3-2.	Dimensions of Common Drill Rods	3-6
Table 3-3.	Dimensions of Common Flush-joint Casings	3-6
Table 3-4.	Common Sampling Methods	3-11
Table 4-1.	Evaluation of the Apparent Density of Coarse-grained Soils	4-12
Table 4-2.	Evaluation of the Consistency of Fine-grained Soils	4-12
Table 4-3.	Adjectives to Describe Water Content of Soils	4-13
Table 4-4.	Particle Size Definition for Gravels and Sands	4-14
Table 4-5.	Adjectives for Describing Size Distribution for Sands and Gravels	4-15
Table 4-6.	Field Methods to Describe Plasticity	4-16
Table 4-7.	Descriptive Terms for Layered Soils	4-18
Table 4-8.	The Unified Classification System	4-19
Table 4-9.	Soil Classification Chart (Laboratory Method)	4-20
Table 4-10.	Soil Plasticity Descriptions	4-24
Table 4-11.	Examples of Description of Fine-grained Soils	4-24
Table 4-12.	AASHTO Soil Classification System	4-27
Table 4-13.	Rock Groups and Types	4-30
Table 4-14.	Terms to Describe Grain Size of (Typically For) Sedimentary Rocks	4-31
Table 4-15.	Terms to Describe Grain Shape (For Sedimentary Rocks)	4-31
Table 4-16.	Terms to Describe Stratum Thickness	4-31
Table 4-17.	Terms to Describe Rock Hardness	4-32
Table 4-18.	Terms to Classify Discontinuities Based on Aperture Size	4-33
Table 5-1.	Relevance of In-situ Tests to Different Soil Types	5-37
Table 6-1.	Field Methods for Measurement of Permeability	6-6
Table 6-2.	Time Intervals for Reading During Pumping Test	6-12
Table 7-1.	Terms in Weight-volume Relations	7-1
Table 7-2.	Unit Weight-volume Relationships	7-2
Table 7-3.	AASHTO And ASTM Standards for Frequently-Used Laboratory Testing of Soils	7-5
Table 7-4.	Common Sense Guidelines for Laboratory Testing of Soils	7-36
Table 7-5.	Summary of Information Needs and Testing Considerations for a Range of Highway Applications	7-38

Table 8-1.	Standards & Procedures for Laboratory Testing of Intact Rock	8-2
Table 8-2.	Common Sense Guidelines for Laboratory Testing of Rocks	8-12
Table 9-1.	Representative Permeability Values for Soils	9-37
Table 10-1.	Primary Rock Types Classified by Geologic Origin	10-2
Table 10-2.	Geologic Time Scale	10-3
Table 10-3.	Representative Range of Dry Unit Weights	10-6
Table 10-4.	Representative Measured Parameters on Intact Rock Specimens	10-8
Table 10-5.	Engineering Classification of Intact Rock	10-11
Table 10-6.	Friction Angles for Rock Joints, Minerals, & Fillings	10-17
Table 10-7.	Residual Friction Angles	10-18
Table 10-8.	Empirical Methods for Evaluating Elastic Modulus (E_M) of Rock Masses	10-27

LIST OF FIGURES

Figure 1-1.	Natural Geomaterials: (a) Atlantic Dune Sand Deposits; (b) Sandstone in Moab, UT	1-1
Figure 2-1.	New Highway Construction: (a) Pile Bent Bridge in NC and (b) Cut Slope in VA.	2-1
Figure 2-2.	Rehabilitation Projects Including: (a) Highway Slope Failure Involving Loss of Life; (b) Roadway Landslide; (c) Sinkhole in Orlando, Florida; and (d) Slope Stabilization	2-2
Figure 2-3.	Example Field Instructions Form for Geotechnical Investigations	2-7
Figure 3-1.	Solid Stem Continuous Flight Auger Drilling System: (a) In use on drill rig, (b) Finger and fishtail bits, (c) Sizes of solid stem auger flights, (d) Different assemblies of bits and auger flights	3-2
Figure 3-2.	Hollow Stem Auger Components	3-3
Figure 3-3.	Hollow Stem Continuous Flight Auger Drilling Systems: (a) Comparison with solid stem auger; (b) Typical drilling configuration; (c) Sizes of hollow stem auger flights; (d) Stepwise center bit; (e) Outer bits; (f) Outer and inner assembly	3-4
Figure 3-4.	Schematic of Drilling Rig for Rotary Wash Methods.	3-5
Figure 3-5.	Rotary Wash Drilling System: (a) Typical drilling configuration; (b) Casing and driving shoe; (c) Diamond, drag, and roller bits; (d) Drill fluid discharge; (e) Fluid cuttings catch screen; (f) Settling basin	3-7
Figure 3-6.	Setup of Bucket Auger & Rig	3-8
Figure 3-7.	Split-Barrel Samplers: (a) Lengths of 457 mm (18 in) and 610 mm (24 in); (b) Inside diameters from 38.1 mm (1.5 in) to 89 mm (3.5 in)	3-12
Figure 3-8.	Split Barrel Sampler: (a) Open sampler with soil sample and cutting shoe; (b) Sample jar, split-spoon, Shelby tube, and storage box for transport of jar samples	3-13
Figure 3-9.	Split Barrel Sampler. (a) Stainless steel and brass retainer rings (b) Sample catchers	3-13
Figure 3-10.	Schematic of Thin-Walled Shelby Tube	3-14
Figure 3-11.	Selected Sizes and Types of Thin-Walled Shelby Tubes	3-14
Figure 3-12.	Shelby Tube Sealing Methods. (a) Microcrystalline wax (b) O-ring packer	3-15
Figure 3-13.	Piston Sampler: (a) Picture with thin-walled tube cut-out to show piston; (b) Schematic	3-16
Figure 3-14.	Pitcher Tube Sampler	3-16
Figure 3-15.	Pitcher Sampler. (a) Sampler Being Lowered into Drill Hole; (b) Sampler During Sampling of Soft Soils; (c) Sampler During Sampling of Stiff or Dense Soils	3-17
Figure 3-16.	Denison Double-Tube Core Barrel Soil Sampler	3-18
Figure 3-17.	(a) Single Tube Core Barrel; (b) Rigid Type Double Tube Core Barrel; (c) Swivel Type Double Tube Core Barrel, Series "M" with Ball Bearings	3-24

Figure 3-18.	Double Tube Core Barrel. (a) Outer barrel assembly (b) Inner barrel assembly	3-25
Figure 3-19.	Coring Bits. From left to right: Diamond, Carbide, & Sawtooth	3-25
Figure 3-20.	Modified Core Recovery as an Index of Rock Mass Quality	3-28
Figure 3-21.	Length Measurement of Core RQD Determination	3-29
Figure 3-22.	Core Box for Storage of Recovered Rock and Labeling	3-30
Figure 3-23.	Rock Formations Showing Joints, Cut Slopes, Planes, and Stabilization Measures	3-31
Figure 3-24.	(a) Structural Mapping Coding Form for Discontinuity Survey Data	3-33
Figure 3-24.	(b) Structural Mapping Coding Form for Slope Assessment	3-24
Figure 3-25.	Views of Rotary Drill Rigs Mounted on Trucks for Soil & Rock Exploration	3-36
Figure 4-1.	Representative Boring Log Form	4-2
Figure 4-2.	Representative Core Boring Log	4-3
Figure 4-3.	Representative Exploration Pit Log	4-4
Figure 4-4.	Proposed Key to Boring Log	4-5
Figure 4-5.	Proposed Key to Core Boring Log	4-7
Figure 4-6.	Flow Chart to Determine the Group Symbol and Group Name for Coarse- grained Soils	4-22
Figure 4-7.	Plasticity Chart for Unified Soil Classification System	4-23
Figure 4-8a.	Flow Chart to Determine the Group Symbol and Group Name for Fine- Grained Soils	4-25
Figure 4-8b.	Flow Chart to Determine the Group Symbol and Group Name for Organic Soils	4-25
Figure 4-9.	Range of Liquid Limit and Plasticity Indices for Soils in Soil Classification Groups A-2, A-4, A-5, A-6 and A-7	4-28
Figure 5-1.	Common In-Situ Tests for Geotechnical Site Characterization of Soils	5-1
Figure 5-2.	Direct-Push Technology: (a) Truck-Mounted and (b) Track-Mounted Cone Rigs	5-2
Figure 5-3.	Sequence of Driving Split-Barrel Sampler During the Standard Penetration Test	5-3
Figure 5-4.	SPT-N values from (a) Uncorrected Data and (b) Corrected to 60% Efficiency	5-4
Figure 5-5.	Various Cone Penetrometers Including Electric Friction and Piezocone Types	5-6
Figure 5-6.	Geometry and Measurements Taken by Cone and Piezocone Penetrometers ..	5-7
Figure 5-7.	Correction Detail for Porewater Pressures Acting on Cone Tip Resistance ..	5-7
Figure 5-8.	Procedures and Components of the Cone Penetration Test	5-8
Figure 5-9.	Piezocene Results next to Mississippi River, Memphis, TN.	5-9
Figure 5-10.	General Test Procedure for the Field Vane in Fine-Grained Soils	5-11
Figure 5-11.	Selection of Vane Shear Blades, Pushing Frames, and Torquemeter Devices	5-12
Figure 5-12.	Definitions of Vane Geometries for Tapered & Rectangular Blades	5-13
Figure 5-13.	Illustrative Results from VSTs Conducted in San Francisco Bay Mud showing Profiles of (a) Peak and Remolded Vane Strengths, and (b) derived Clay Sensitivity	5-14

Figure 5-14.	Vane Correction Factor ($:_R$) Expressed in Terms of Plasticity Index and Time to Failure	5-15
Figure 5-15.	Setup and Sequence of Procedures for the Flat Plate Dilatometer Test	5-17
Figure 5-16.	Flat Plate Dilatometer Equipment: (a) Modern Dual-Element Gauge System; (b) Early Single-Gauge Readout; (c) Computerized Data Acquisition Model	5-18
Figure 5-17.	Example DMT Sounding in Piedmont residual soils (CL to ML) in Charlotte, NC.	5-18
Figure 5-18	Test Procedure and Conduct of the Pre-Bored Type (Menard) Pressuremeter Test	5-20
Figure 5-19.	Photos of Pressuremeter Equipment, including Menard-type pressure panel, SBP probe, SBP cutter teeth, hydraulic jack, and monocell-type probe	5-21
Figure 5-20.	Menard-type Pressuremeter Results for Utah DOT Project	5-22
Figure 5-21.	Representative Compression Wave Velocities of Various Soil and Rock Materials	5-24
Figure 5-22.	Representative Shear Wave Velocities of Various Soil and Rock Materials .	5-24
Figure 5-23.	Field Setup & Procedures for Seismic Refraction Method	5-25
Figure 5-24.	Data Reduction of SR Measurements to Determine Depth to Hard Layer .	5-26
Figure 5-25.	Setup and Data Reduction Procedures for Crosshole Seismic Test	5-27
Figure 5-26.	Setup and Data Reduction Procedures for Conducting a Downhole Seismic Survey	5-28
Figure 5-27.	Summary Shear Wave Trains from Downhole Tests at Mud Island, Memphis, TN.	5-28
Figure 5-28.	Results of Seismic Piezocone Sounding in Residual Soils in Coweta County, Georgia showing four independent readings with depth.	5-29
Figure 5-29.	Field Setup for Conducting Spectral Analysis of Surface Waves (SASW) .	5-30
Figure 5-30.	Spectrum Analyzer and Data Logging Equipment for SASW	5-30
Figure 5-31.	Comparison of Shear Wave Profiles from Different Geophysical Techniques	5-31
Figure 5-32.	Ground Penetrating Radar (GPR) Equipment from Xadar, GeoVision, and EKKO Sensors & Software	5-32
Figure 5-33.	GPR Results: (a) Buried Utility Locations; (b) Soil Profile of Fill over Soil; and (c) GPR Locating of Underground Tanks and Pipes	5-33
Figure 5-34.	Representative Values of Resistivity for Different Geomaterials	5-34
Figure 5-35.	Electrical Resistivity Equipment and Results: (a) Oyo System; (b) Advanced Geosciences Inc.; (c) Two-Dimensional Cross-Section Resistivity Profile for Detection of Sinkholes and Caves in Limestone	5-35
Figure 5-36.	EM Survey to Detect Underground Storage Tanks	5-35
Figure 5-37.	Magnetometer Survey Results	5-36
Figure 6-1:	Representative Details of Observation Well Installations. (a) Drilled-in-place Stand-Pipe Piezometer, (b) Driven Well Point	6-3
Figure 6-2:	Packer-Type Pressure-Test Apparatus for Determining the Permeability of Rock. (a) Schematic Diagram; (b) Detail of Packer Unit	6-8
Figure 6-3.	A General Configuration and Layout of Piezometers for a Pumping Test .	6-11
Figure 6-4.	Drawdown in an Observation Well Versus Pumping Time	6-13

Figure 6-5.	Definitions of Terms in Pumping Test Within an Unconfined Aquifer	6-14
Figure 6-6.	Definitions of Terms in Pumping Test Within a Confined Aquifer System . .	6-14
Figure 6-7.	General Procedure for Slug Test in a Screened Borehole	6-15
Figure 6-8.	Porewater Pressure Dissipation Response in Soft Varved Clay at Amherst NGES	6-17
Figure 6-9.	Coefficient of Permeability (k = Hydraulic Conductivity) from Measured Time to 50% Consolidation (t_{50}) for Monotonic Type 2 Piezocone Dissipation Tests	6-18
Figure 7-1.	Laboratory Sieves for Mechanical Analysis for Grain Size Distributions . .	7-9
Figure 7-2.	Representative Grain Size Curves for Several Soil Types	7-10
Figure 7-3.	Liquid Limit Test by (a) Manual Casagrande Cup Device; (b) Electric Fall Cone	7-12
Figure 7-4.	A Representative Moisture-Density Relationship from a Standard Compaction Test	7-12
Figure 7-5.	Definitions of Effective Stress Parameters For Mohr-Coulomb Failure Criterion	7-17
Figure 7-6.	Measured Stress-Strain for Unconfined Compressive Test	7-18
Figure 7-7.	Triaxial Test Apparatuses and Equipment	7-20
Figure 7-8.	Effective Stress Mohr Circles for Consolidated Undrained Triaxial Tests .	7-21
Figure 7-9.	Effective $q-p'$ Strength Envelopes for Consolidated Undrained Triaxial Tests	7-21
Figure 7-10.	Direct Shear Test Devices	7-23
Figure 7-11.	Illustrative Results from DS Tests on Clay Involved in Route 1 Slope Stability Study, Raleigh, NC	7-23
Figure 7-12.	Resonant Column Test (RCT) Equipment for Determining G_{max} and D in Soils	7-24
Figure 7-13.	Results from Resonant Column Testing of Light Castle Sand	7-25
Figure 7-14.	Permeability Test Schematics: (a) Constant Head Device; (b) Falling Head Test	7-29
Figure 7-15.	Permeameter Equipment: (a) Flexible-Walled Permeameter Cell; (b) Permeability Station with Automatic Volume Change Device (left) and Backpressure Panel Board	7-30
Figure 7-16.	One Dimensional Consolidation Devices and Results	7-32
Figure 8-0.	(a) Intact Rock Specimens for Laboratory Testing; (b) Compressive Strength Testing	8-1
Figure 9-1.	Delineation of Geostratigraphy and Soil & Rock Types by Drill & Sampling Methods	9-3
Figure 9-2.	Factors Affecting Cone Penetrometer Test Measurements in Soils	9-4
Figure 9-3.	Chart for Soil Behavioral Classification by CPT	9-5
Figure 9-4.	Interrelationship Between Saturated Unit Weight and In-Place Water Content of Geomaterials	9-7

Figure 9-5.	Unit Weight Relationship with Shear Wave Velocity and Depth in Saturated Geomaterials	9-8
Figure 9-6.	Interrelationship Between Minimum and Maximum Dry Densities of Quartz Sands	9-9
Figure 9-7.	Maximum Dry Density Relationship with Sand Uniformity Coefficient	9-10
Figure 9-8.	Relative Density of Clean Sands from Standard Penetration Test Data	9-11
Figure 9-9.	Relative Density Evaluations of NC and OC Clean Quartz Sands from CPT Data	9-12
Figure 9-10.	Relative Density of Clean Sands Versus DMT Horizontal Stress Index	9-12
Figure 9-11.	Typical Values of N_r and Unit Weight for Cohesionless Soils	9-13
Figure 9-12.	Peak Friction Angle of Sands from SPT Resistance	9-14
Figure 9-13.	Peak Friction Angle of Unaged Clean Quartz Sands from Normalized CPT Tip Resistance	9-15
Figure 9-14.	Evaluation of Peak Friction Angle of Sands from DMT Results Based on Wedge-Plasticity Solutions and Experimental Data	9-15
Figure 9-15.	Processing of PMT Data in Sands for Peak N_r Determination	9-16
Figure 9-16.	Relation Between Peak N_r for Clean Sands and Slope Parameter (s) from PMT Data	9-16
Figure 9-17.	Representative Consolidation Test Results in Overconsolidated Clay	9-17
Figure 9-18.	Trends for Compression and Swelling Indices in Terms of Plasticity Index ..	9-18
Figure 9-19.	Ratio of Measured Vane Strength to Preconsolidation Stress ($s_{uv}/F_p r$) vs. Plasticity Index (I_p)	9-18
Figure 9-20.	Preconsolidation Stress Relationship with Net Cone Tip Resistance from Electrical CPT	9-19
Figure 9-21.	Relationship Between Preconsolidation Stress and Excess Porewater Pressures from Piezocones	9-19
Figure 9-22.	Relationship Between Preconsolidation Stress and DMT Effective Contact Pressure in Clays	9-20
Figure 9-23.	Relationship Between Preconsolidation Stress and Shear Wave Velocity in Clays	9-20
Figure 9-24.	Relationships Between Overconsolidation Ratio and DMT Horizontal Stress Index, K_D from (a) Cavity Expansion-Critical State Theory, and (b) Worldwide Database from Clays	9-21
Figure 9-25.	Summary Calibrations of OCR Evaluations Using Piezocone Results in Clays with Superimposed Curves from Analytical Model	9-22
Figure 9-26.	Modes of Undrained Shear Strength Ratio for Normally-Consolidated Clays ..	9-23
Figure 9-27.	Normalized Undrained Strengths for NC Clay Under Different Loading Modes by Constitutive Model	9-23
Figure 9-28.	Undrained Strength Ratio Relationship with OCR and N_r for Simple Shear Mode	9-24
Figure 9-29.	Field K_0 - OCR Relationships for (a) Natural Clays and (b) Natural Sands ..	9-25
Figure 9-30.	Relationship for Lateral Stress State Determination in Sands from CPT	9-26
Figure 9-31.	Definitions of Elastic Moduli in Terms of Loading & Applied Boundary Conditions	9-27

Figure 9-32.	Idealized Stress-Strain Curve and Stiffnesses of Soils at Small- and Large-Strains	9-28
Figure 9-33.	Conceptual Variation of Shear Modulus with Strain Level Under Static Monotonic Loading and Relevance to In-Situ Tests	9-29
Figure 9-34.	Modulus Reduction with Log Shear Strain for Initial Monotonic (Static) and Dynamic (Cyclic) Loading Conditions	9-30
Figure 9-35.	Modulus Degradation from Instrumented Laboratory Tests on Uncemented and Unstructured Geomaterials	9-31
Figure 9-36.	Modified Hyperbolas to Illustrate Modulus Degradation Curves	9-32
Figure 9-37.	Results of Seismic Piezocone Tests (SCPTu) in Layered Soil Profile, Wolf River, Memphis, TN	9-33
Figure 9-38.	Ratio of G_0/q_c with Normalized CPT Resistance for Uncemented Sands	9-35
Figure 9-39.	Ratio of G_0/E_D with Normalized DMT Reading for Clean Quartz Sands	9-34
Figure 9-40.	Trend Between G_0 and CPT Tip Stress q_T in Clay Soils	9-34
Figure 9-41.	Trend Between G_0 and DMT modulus E_D in Clay Soils	9-35
Figure 9-42.	Modulus (D') vs. Shear Modulus (G_0) in Clays	9-35
Figure 9-43a.	Modified Time Factors for u_1 Monotonic Porewater Dissipations	9-38
Figure 9-43b.	Modified Time Factors for u_2 Monotonic Porewater Dissipations	9-38
Figure 9-44.	Estimation of Rigidity Index from OCR and Plasticity Index	9-39
Figure 9-45	Coefficient of Consolidation for 50% Dissipation from Shoulder Readings	9-40
Figure 9-46.	Representative Solutions for Type 2 Dilatory Dissipation Curves at Various OCRs	
Figure 10-1.	Generalized Distribution of Sedimentary, Igneous, & Metamorphic Rocks in the U.S.A	10-1
Figure 10-2.	Factors & Parameters Affecting Geologic Mapping of Rock Mass Features	10-4
Figure 10-3.	Specific Gravity of Solids for Selected Rock Minerals	10-5
Figure 10-4.	Saturated Rock Unit Weight in Terms of Porosity and Specific Gravity	10-7
Figure 10-5.	Representative S- and P-wave velocities for Intact Rock Materials	10-7
Figure 10-6.	Classifications for Unweathered Intact Rock Material Strength	10-9
Figure 10-7.	Interrelationship Between Uniaxial Compression, Triaxial, and Tensile Strength of Intact Rock in Mohr-Coulomb Diagram	10-10
Figure 10-8.	Comparison of Tensile vs. Compressive Strengths for Intact Rock Specimens	10-10
Figure 10-9a.	Elastic Modulus-Compressive Strength Groupings for Intact Igneous Rock Materials	10-12
Figure 10-9b.	Elastic Modulus-Compressive Strength Groupings for Intact Sedimentary Rock Materials	10-13
Figure 10-9c.	Elastic Modulus-Compressive Strength Groupings for Intact Metamorphic Rock Materials	10-14
Figure 10-10.	Small-Strain Elastic Modulus (E_{max}) versus Compressive Strength (q_u) for All Types of Civil Engineering Materials	10-15
Figure 10-11.	Illustrative Cases for Defining Rock Shear Strength for Cut, including: (a) intact rock strength, (b) intact strength across joints, (c) shear strength along joint planes, and (d) jointed rock mass	10-16
Figure 10-12.	Selection of Exposed Rock Masses from Different Geologic Origins	10-19

Figure 10-13. The Geomechanics Classification System for Rock Mass Rating (RMR)	10-21
Figure 10-14. The Q-Rating System for Rock Mass Classification	10-22
Figure 10-15. Chart for Estimating the Geological Strength Index (GSI)	10-23
Figure 10-16. Material Constant m_i for GSI Evaluation of Rock Mass Strength	10-25
Figure 10-17. Approximate Chart Solution for Obtaining Normalized Cohesion Intercept (c_r/F_u) and Friction Angle (Nr) from GSI Rating and m_i Parameter	10-26
Figure 10-18. Allowable Bearing Stresses on Unweathered Rock from Codes	10-28
Figure 10-19. Allowable Bearing Stress on Fractured Rock from RQD	10-28
Figure 10-20. Unit Side Resistance Trend with Strength of Sedimentary Rocks	10-29
Figure 10-21. Shaft Unit Side Resistance with Various Rock Types	10-29
Figure 10-22. Rippability of Inplace Rock by Caterpillar Dozer Evaluate by P-Wave Velocity	10-30
Figure 11-1. Example <i>Table of Contents</i> for a Geotechnical Investigation (Data) Report . .	11-3
Figure 11-2. Example <i>Table of Contents</i> for a Geotechnical Design Report	11-4
Figure 11-3. Representative Test Location Plan of Completed Soil Boring Locations . .	11-7
Figure 11-4. Plan Showing Proposed Boring and In-Situ Test Locations	11-7
Figure 11-5. Subsurface Profile Based on Boring Data Showing Cross-Sectional View . .	11-8
Figure 12-1. Track-Mounted Drill Rig Investigating Bridge Site in Hayti, Missouri . . .	12-2

[Blank]

Nomenclature & Symbols

$"_j$	Joint dip direction
$"_s$	Slope dip direction
$\$$	Average dip angle of rock bedding
$\$_j$	Joint dip
$\$_s$	Slope dip
$(^)$	Buoyant (or effective or submerged) unit weight of geomaterial
$(^)$	Unit weight of soil
$(^{dry})$	Dry unit weight of soil
$(^{dmax})$	Dry unit weight of soil in its densest state
$(^{dmin})$	Dry unit weight of soil in its loosest state
$(^{sat})$	Saturated unit weight of soil
$(^t)$	Total unit weight of soil
$(^w)$	Unit weight of water (9.81 kN/m^3)
$*$	Horizontal movement of soil mass in a Direct Shear Test
$)_{,a}$	Change in axial strain
$)_F$	Change in applied axial stress
$)_D$	Change in diameter of rock sample
$)_e$	Change in void ratio over p
$)_H$	Vertical movement of soil mass in a Direct Shear Test
$)_H$	Change in height of rock sample
$)_p$	Additional loading due to foundation or embankment construction
$)_t$	Time for standpipe head to fall
$'_{ax}$	Axial strain in soil or rock sample (H/H)
$'_{radial}$	Radial strain in rock sample (D/D)
:	Viscosity of the permeant
$:_{FV}$	Correction factor for vane shear strength to mobilized strength
<	Poisson's ratio
D	Resistivity; $= 2BdV/I$
F'	Effective stress
F	Normal stress
F_1, F_2, F_3	Major, intermediate and minor total principal stresses, respectively.
F_1r, F_2r, F_3r	Major, intermediate and minor effective principal stresses
$F_{a(ult)}$	Uniaxial compressive strength of rock
F_{CIR}	Uniaxial compressive strength of Intact Rock
F_n	Normal stress on joint
F_u	Applied axial stress
F_v	Total overburden pressure
F_{vo}	Total (vertical) overburden stress
$F_{vo}r$	Effective (vertical) overburden stress
J	Shear stress
$(J_u)_{corr}$	Corrected vane shear strength
$(J_u)_{field}$	Vane shear strength measured in the field (uncorrected)
N'	Drained or effective friction angle of soil or rock

N	Angle of internal friction
N_d	Drained friction angle
N_r	Residual friction angle
A	Uncorrected pressure required to cause flat dilatometer diaphragm to just lift-off
A	Loaded area; Cross-sectional area of soil sample
A	Code for Auger sample to be entered in the “Samples Type” column of boring log
AASHTO	American Association of State Highway and Transportation Officials
ADSC	Association of Drilled Shaft Contractors
AQ Wireline	Designation of rock core barrel
ASTM	American Society for Testing and Materials
B	Bedding (used to describe type of discontinuity in rock core log)
B	Uncorrected pressure for 1.1 mm deflection of flat dilatometer membrane.
B_f	Width of footing
BHS	Code for Borehole shear test to be entered in the column of boring log
BQ	Dimension of rock core size
BX	Rock cored with BX core barrel, which obtains a 41 mm-diameter core
C	Code for Denison or pitcher-type core barrel sample
C	Code for consolidation test for “Samples Type” column of boring log
C	Close (used to describe discontinuity spacing in rock core log)
C	Uncorrected pressure during deflation of flat plate dilatometer membrane.
c	Shape factor
c'	Drained or effective cohesion intercept of soil or rock from drained lab shear test.
C_s	Coefficient of secondary consolidation
C_s'	Coefficient of secondary compression in terms of strain
C_s^e	Coefficient of secondary compression in terms of void ratio
C_1	Hazen’s coefficient
Ca	Calcite (used to describe type of infilling in rock core log)
CBR	California Bearing Ratio
C_c	Coefficient of curvature
C_c	(Virgin) Compression index
CD	Consolidated Drained
CDS	Completely Decomposed State
CH	Inorganic clays of high plasticity
Ch	Chlorite (used to describe type of infilling in rock core log)
c_h	Coefficient of horizontal consolidation
CL	Inorganic clays of low to medium plasticity
Cl	Clay (used to describe type of infilling in rock core log)
c_o	Cohesion of as-compacted soil
CP	Designation of rock core barrel
CPT	Cone Penetration Test
CR	Compression Ratio = $C_c/(1+e)$
C_r	Recompression Index
C_u	Uniformity coefficient; = D_{60}/D_{10}
CU	Consolidated Undrained (Triaxial shear test)
c_u	Undrained shear strength
c_v	Coefficient of vertical consolidation

D	Original diameter of rock sample
D	Apparent diameter of the soil particles
d	Primary consolidation at a specific load level
d	Depth
d	Distance between electrodes in resistivity survey.
D_{10}	Grain size than which 10% of the sample is smaller
D_{30}	Grain size than which 30% of the sample is smaller
D_{50}	Mean Grain Size; size than which 50% of the sample is finer
D_{60}	Grain size than which 60% of the sample is smaller
D_{\max}	Largest grain size in soil sample
D_{\min}	Smallest grain size in soil sample
DMT	Flat plate dilatometer test
D_r	Relative density of soil
DS	Code for direct shear test to be entered in the "Other Tests" column of boring log
D_s	Effective particle diameter
DSS	Direct Simple Shear
E	Elastic or Young's Modulus
e	Void ratio of soil
E_{av}	Average Young's Modulus
E_D	Equivalent elastic modulus obtained from flat dilatometer.
e_f	Final void ratio
E_M	Menard modulus from standard (prebored) pressuremeter test.
E_m	In-situ modulus of deformation
e_{\max}	Void ratio of soil in its loosest state
e_{\min}	Void ratio of soil in its densest state
e_o	Initial void ratio of sample
e_r	Void ratio at beginning of rebound
EROS	Earth Resources Observations Systems
E_s	Secant Young's Modulus
E_t	Tangent Young's Modulus
EW	Designation of flush-joint casing
EX	Designation of rock core barrel
F	Friable (term to describe rock hardness)
F	Fault (used to describe type of discontinuity in rock core log)
F	Fines; Corresponding to percent soil passing No. 200 sieve
f	Shear wave frequency
Fe	Iron oxide (used to describe type of infilling in rock core log)
Fi	Filled (used to describe amount of infilling in rock core log)
Fo	Foliation (used to describe type of discontinuity in rock core log)
f_s	Measured sleeve friction during CPT
FV	Field Vane or Vane Shear Test
GC	Clayey gravels, poorly graded gravel-sand-clay
GI	Group index in the AASHTO soil classification system
GM	Silty gravels, poorly graded gravel-sand-silt
GP	Poorly graded clean gravels, gravel-sand mixture
GPR	Ground Penetrating Radar

G_s	Specific gravity of soil solids
GW	Well graded clean gravels, gravel-sand mixture
Gy	Gypsum/Talc (used to describe a special type of infilling in rock core log)
H	High modulus ratio
H	Healed (used to describe type of infilling in rock core log)
H	Differential head of pressure on the test section
H	Hard (term to describe rock hardness)
H	Half height of consolidation sample (Length of longest drainage path)
H	Original height of rock sample
h_1, h_2	Heads at times t_1 and t_2 , respectively
HQ	Dimension of rock core size
HW	Designation of drill rod
i	Angle of irregularities with average dip line
$I_{a(50)}$	Anisotropic point load strength index of rock specimen
I_D	Material index for obtaining soil type from flat plate dilatometer test.
I_{d2}	Slake-Durability Index
I_p, PI	Plasticity Index
Ir	Irregular (used to describe surface shape of joint in rock core log)
I_s	Point-load index
$I_{s(50)}$	Point load strength index of rock specimen with diameter = 50 mm
ISRM	International Society for Rock Mechanics
J	Joint (used to describe type of discontinuity in rock core log)
J_a	Joint alteration number in the Q System
JCS	Joint wall Compressive Strength
J_r	Joint roughness coefficient in the Q System
JRS	Joint Roughness Coefficient
J_v	Number of joints in unit volume of rock
k	Coefficient of permeability (hydraulic conductivity)
K_D	Lateral stress index from flat dilatometer.
K_o	Lateral stress coefficient for geostatic case.
L	Length of soil sample
L	Low modulus ratio
L_f	Length of footing
LFC	Length of fully cylindrical rock core piece
LH	Low hardness (term to describe rock hardness)
LI	Liquidity Index
LL	Liquid Limit
LPS	Latent Planes of Separation
LT	Length of rock core piece measured from tip to tip
M	Moderate (used to describe discontinuity spacing in rock core log)
M	Average modulus ratio
M	Mechanical (sieve or hydrometer) analysis
MFS	Micro Fresh State
MH	Inorganic clayey silts, elastic silts
MH	Moderately Hard (term to describe rock hardness)

ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts (Group symbol in Unified Soil Classifications System)
ML-CL	Mixtures of inorganic silts and clays
MW	Moderately wide (used to describe discontinuity width in rock core log)
N	Uncorrected Standard Penetration Test N-value (or blow counts).
n	Porosity
N_1	N-value normalized to an effective overburden stress of 1 atmosphere
N_{60}	SPT N-value corrected for energy to average 60% standard of practice.
$(N_1)_{60}$	SPT N-value corrected to 60% energy efficient and stress-normalized.
NC	Normally Consolidated
N_{corr}	N-value of saturated fine or silty sands corrected for pore pressure
N_{field}	N-value measured in the field
NGI	Norwegian Geotechnical Institute
No	None (used to describe amount or type of infilling in rock core log)
NQ	Dimension of rock core size
NR	No recovery of sample
NV	Designation of rock core barrel
NW	Designation of drill rod
NX	Rock cored with NX core barrel, which obtains a 53 mm-diameter core
OC	Overconsolidated
OCR	Overconsolidation Ratio
OH	Organic clays of medium to high plasticity, organic salts (Group symbol in Unified Soil Classifications System)
OL	Organic silts and organic silty clays of low plasticity (Group symbol in Unified Soil Classifications System)
OMC	Optimum Moisture Content
P	Piezometer
P	Code for thin-wall tube sample in the “Samples Type” column of boring log
p_1	Pressure B corrected for diaphragm stiffness in flat dilatometer test.
Pa	Partially filled (used to describe amount of infilling in rock core log)
p_c	Preconsolidation stress
PDS	Partly Decomposed State
p_f	Creep pressure during Menard-type pressuremeter test
PI	= LL - PL ; Plasticity index
PL	Plastic Limit
p_l	Limit pressure during Menard-type pressuremeter test
PLT	Point Load Test
PMT	Pressuremeter Test
P_o	Pressure corresponding to volume V_o during Menard-type pressuremeter test
p_o	Pressure A corrected for diaphragm stiffness in flat dilatometer tes.
PQ	Dimension of rock core size
Ps	Code for piston sample to be entered in the “Samples Type” column of boring log
Pt	Peat and other highly organic soils
PVC	Poly-vinyl chloride
PW	Designation of flush-joint casing
Py	Pyrite (used to describe type of infilling in rock core log)

Q	Constant rate of flow of water into the hole; Total discharge volume
q_c	Uncorrected cone tip resistance measured during CPT
q_t	Corrected cone tip stress or resistance during CPT
q_u	Unconfined compressive strength; Uniaxial compressive strength of rock
Qz	Quartz (used to describe type of infilling in rock core log)
R	Rough (used to describe roughness of surface in rock core log)
R	Shale rating
r	Radius of the test borehole
R-value	Value of resistance of the soil to lateral deformation when a vertical load acts on it
RMR	Rock Mass Rating
RQD	Rock Quality Designation
RR	Recompression Ratio = $C_r/(1+e)$
RW	Designation of drill rod
RW	Designation of flush-joint casing
S	Degree of saturation of soil
S	Smooth (used to describe roughness of surface in rock core log)
SC	Clayey sands, poorly graded sand-clay mixture
Sd	Sand (used to describe type of infilling in rock core log)
SDI	Slake Durability Index
Sh	Shear (used to describe type of discontinuity in rock core log)
SL	Shrinkage limit
Slk	Slickensided (used to describe roughness of surface in rock core log)
SM	Silty sands, poorly graded sand-silt mixture
SM-SC	Sand-silt-clay with slightly plastic fines
SMR	Slope rock Mass Rating
SP	Poorly graded clean sands, sand-gravel mixture
Sp	Spotty (used to describe amount of infilling in rock core log)
SPB	Preferred Breakage
SPT	Standard Penetration Test
SR	Slightly rough (used to describe roughness of surface in rock core log)
SRB	Random Breakage
SRS	Shale Rating System
SS	Code for standard spoon sample in the “Samples Type” column of boring log
St	Stepped (used to describe surface shape of joint in rock core log)
STS	Stained State
Su	Surface stain (used to describe amount of infilling in rock core log)
s_u	Undrained shear strength
s_{uv}	Vane shear strength (uncorrected)
$s_u/F_{vo}r$	Normalized undrained shear strength to effective overburden stress ratio.
SW	Well-graded sands, gravelly sands, little or no fines (Group symbol in USCS).
SW	Designation of flush-joint casing
T	Code for triaxial compression test in the “Other Tests” column of boring log
T	Topping failure; Tight (used to describe discontinuity width in rock core log)
T	Shear force on soil in a Direct Shear Test
t	Time
t_{100}	Time required for 100% consolidation at a specific load level

t_{50}	Time required for 50% consolidation at a specific load level
TV	Code for torvane index in the “Other Tests” column of boring log
U	Code for unconfined compression test in the “Other Tests” column of boring log
u	Porewater pressure
u_1	Porewater pressure during type 1 piezocone (midface element)
u_2	Porewater pressure during type 2 piezocone (shoulder element)
u_o	In-situ hydrostatic porewater pressure
USCS	Unified Soil Classification System
UU	Unconsolidated Undrained
UW	Designation of flush-joint casing
V	Potential drop in resistivity surveys
V	Vein (used to describe type of discontinuity in rock core log)
VC	Very close (used to describe discontinuity spacing in rock core log)
V_c	Initial volume of probe during Menard’s pressuremeter test
V_f	volume corresponding to creep pressure p_f during Menard’s pressuremeter test
VH	Very hard (term to describe rock hardness)
V_m	$(V_o + V_f)$ during Menard pressuremeter test
VN	Very narrow (used to describe discontinuity width in rock core log)
v_o	Difference between the volume of the hole and V_c
VR	Very rough (used to describe roughness of surface in rock core log)
V_s	Shear wave velocity
W	Wide (used to describe discontinuity width in rock core log)
W	Code for unit weight and water content in the “Other Tests” column of boring log
w	Natural moisture content
Wa	Wavy (used to describe surface shape of joint in rock core log)
W_n	Natural water content
X	Distance
X	Code for special tests performed in the “Other Tests” column of boring log
ZW	Designation of flush-joint casing
z	Depth (below ground)

[Blank]